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MEMORANDUM

To: Jeff Shea, P.E. **Date:** April 30, 2021

From: Guy Hesketh, P.E.

Subject: Response to Engineering Comments Talcott Mountain Self Storage
Our File: 21126

Please find responses to comments of you April 27, 2012. Your comments in normal font, our responses in **bold font**.

1. There is a slight discrepancy in the rainfall values in the NOAA Atlas 14 "Point Precipitation Frequency Estimates" in Attachment 1, the Hydrographs in Attachment 3, and the Hydraflow Rainfall Report in Attachment 3. Although these discrepancies are minor, check and adjust the rainfall values to reflect the NOAA Atlas 14 rainfall data.

The NOAA data is downloaded off the NOAA Website and input into the Hydraflow program in a tabular form. The program then uses the input data to generate a curvilinear formula from the NOAA data to generate a "best fit" IDF (Intensity-Duration-Frequency) curve". It appears that the program then takes the best fit curve to produce a chart to show the tabular Intensity Values (in/hr) on the Hydraflow Rainfall Report - Intensity. The tabular intensity values do not often match the NOAA input data, one for one, as rounding errors are generated by the process. A copy of the Storm Sewer IDF Curve, Hydraflow Rainfall Report, and NOAA Raw data download (inches per hour) are attached. This minor discrepancy does not negate the analysis.

2. The bottom elevation of the Water Quality Basin is 190.0' whereas the existing grade in this area is higher than 195.0'. The cut of greater than 5 feet may encounter groundwater and therefore reduce the amount of storage available within the basin.

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Provide test pit data in this location to confirm depth to groundwater. Testing should be witness by a qualified professional licensed in the State of Connecticut.

The excavation of test pits and infiltration testing will be conducted on Monday, May 3, under the direction of a qualified and licensed professional engineer. A total of four test pits will be excavated at select, representative locations of the site and to one to two feet below the bottom of the elevation of the infiltration basin and infiltration galleys to assess subsurface soil conditions and determine the seasonal high groundwater levels. Stand-pipes will be placed in the excavations prior to backfilling and following backfilling will be used to conduct in-situ falling head tests. The standpipes will consist of 3-inch diameter, solid-wall PVC pipe. The bottoms of the pipes will be driven into the un-disturbed soil at the bottom of the excavation and the holes backfilled. The pipes will be filled with water and the fall rate of the water measures to assess the relative permeability of the soil. Soil logs and results of the infiltration testing will be provided to the Town Engineer following completion.

3. Conform to Section 5.2.1.g. of the Highway Construction and Design Standards for the Town of Simsbury – Minimum Cover: A minimum cover of 2.5 feet shall be provided for all drain pipes unless special designs, as approved by the Town Engineer, are utilized. Several pipes are proposed with significantly less than 2.5 feet of cover, some with less than 1 foot of cover.

The proposed design specifies a minimum of 1 foot of cover over storm drain piping in all paved areas where CPE (N12) pipe is specified. The manufacturer of this pipe certifies that N12 pipe will support H-25 loads with 12" minimum cover. In cases where it appears that less than 12 inches of cover are provided, it should be noted that the top-of-frame elevation depicted on the catch basin includes the 2-inch drop of the inlet grate (relative to the curbline elevation) Excluding the 2-inch drop, the minimum of one foot of cover is provided. Further noted, the CPE pipe runs have been placed along the curb line where they are not exposed to vehicle traffic.

In the case where RCP pipe is specified, and where one foot of cover is depicted, Class V RCP pipe will be used. A minimum of one foot of cover is recognized as acceptable for Class V pipe. The call-out for the 12" & 24" RCP in the drive has been revised to Class V pipe.

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4. The Hydraflow Pond Report shows a total storage from elevation 190.00 to 194.00 of 26,287 cubic feet. The Water Quality Volume Size Calculations sheet shows a total provided storage from elevation 190.0 to 194.0 of 11,914 cubic feet. Please confirm the total storage of the Water Quality Basin and maintain the consistency across all calculations.

The Water Quality Volume (WQV) presented in the Attachment 3 of the April 6, 2021 Stormwater Management Report was erroneous. Calculations have been revised to show the water quality volume in the water quality basin between elevation 190 (bottom of basin elevation) and elevation 193 (the outfall elevation). The revised Water Quality Volume Size Calculations spreadsheet is attached. The calculated water quality volume provided in the spreadsheet is 17,620 cubic feet. This varies slightly from the total volume of storage provided between elevation 190 and 193 in the Pond Report on Page 14 of the Hydraflow Report (Attachment 3 of the April 6, 2021 Stormwater Management Report) due to the program using the conical method (program), as opposed to the average end area method (spreadsheet), although the values are equally precise to three significant figures. The total volume of the basin from elevation 190 to 194 is not representative of the water quality volume since the water quality volume is that below elevation 193, the outlet elevation.

5. Based on the 2004 Connecticut Stormwater Quality Manual (CTSWQM), the proposed "Water Quality Basin" is considered a "Dry Detention Pond" as proposed. Dry Detention Basins are considered a Conventional Secondary Treatment Practice due to their water quantity control, but limited applicability for water quality control. Provide rationale for why the practice meets the performance standards required for classification as a primary treatment practice, as stated on page 11-3 of the CTSWQM. Alternatively, a sediment forebay enhancement should be considered to improve pretreatment of site runoff prior to entering infiltration / detention basin. It appears there may be adequate capacity in the basin to accommodate this enhancement.

The proposed Water Quality Basin is designed as an Infiltration Basin, which per Chapter 11 of the CTSWQM is a primary treatment practice. The basin is designed to capture and infiltrate 100% of the water quality volume before there is a surface water release. Dry Detention Basins are defined as those that have outlets at the bottom of the pond and allow for release prior to complete detention of the WQV. Our design does not meet the "Dry Detention Basin" criteria since the outfall is not at the bottom of the basin and the basin is

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designed to capture and treat 100% of the WQV prior to release from the basin. The proposed detention/infiltration basin is designed with a sediment forebay that has a volumetric capacity of 1,575 cubic feet. This is equivalent to 32% of the calculated minimum-recommended WQV (4,888 cubic feet), and is in accordance with the requirement that the sediment forebay be sized to accommodate a minimum of 25% of the minimum-recommended WQV, per Table II-P3-2 of the CTSWQM. See the revised Water Quality Volume Size Calculations spreadsheet (attached).

6. A stone check dam is proposed within the infiltration / detention basin. Confirm whether this is intended to remain within the basin post-construction.

The stone check dam will remain following construction and provide for separation of the sediment forebay and primary body of the water quality basin.

7. Labels for the subsurface chambers state "Bottom El.= ". Clarify whether this is the bottom elevation of the chamber or bottom elevation at the bottom of 6" stone bed beneath the chamber.

The "Bottom El." is intended to indicate the bottom elevation of the chamber structure and not the 6-inch stone layer below. The detail on Sheet SD-2 was revised to show clarity.

8. Add check dams in the proposed swale at the northeastern most corner of the site.

Temporary erosion control measures are depicted in the swale and will be maintained until the swale is stabilized with vegetative growth. The minor volumes of water anticipated to flow in the swale do not warrant stone check dams. The design intent is to provide a gentle swale that can be easily manicured with the balance of the lawn following establishment of lawn.

9. There is an "ECB" label on Sheet EC-1 which appears to be a slope protection, although there is no ECB in the legend or in the detail sheets. Provide more detail on what is required for slope protection.

ECB is an acronym for Erosion Control Blanket. Sheet EC-1 has been revised to include ECB in the legend. Additional notes and shading have been provided on Sheet EC-1 to depict areas where ECB should be installed. The detail in Sheet

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SD-1 was revised from Erosion Control Fabric to Erosion Control Blanket, for clarity.

10. Utility Notes #11 and #17 on Sheet UT-1 references Aquarian. #34 Hopmeadow Street is currently serviced by Avon Water Co. according to Town of Simsbury records.

Avon Water Co. is now owned by CT Water. CTW has notified us they do not service this far north on Route 10, it is in Aquarian's service area. We are in communication with Aquarian.

11. Approval will be required by WPCA for connection to the sewer manhole off-site.

We are in communication with WPCA regarding the proposed connection.

12. Provide hoods in all catch basins to trap grease, oils, debris, etc. prior to conveyance into the detention basin.

Trap hoods are proposed in the last basins of the two storm drain systems prior to discharge into the water quality basin. These basins will be specified with 4-foot-deep sumps. In addition, all proposed catch basins will be specified with 4-foot-deep sumps. The small catchment areas of the basins do not warrant the use of trap hoods for all basins. The deeper sumps will provide adequate separation of sediments, and the trap hoods at the last basins will provide adequate entrapment of floatables.

13. The Accessible Parking Space Layout Detail shows one accessible space and one van accessible space with accessible aisles on either side. This does not reflect what is proposed on the plan view.

The detail for the Accessible Parking Space has been revised to show only the van-accessible space, to prevent confusion.

14. Include on the final project plans a long-term maintenance plan for the stormwater management system.

A long-term maintenance plan for stormwater management has been added to Sheet NT-1.

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15. Provide calculations that confirm WQV will drain from the detention basin within 48 -72 hours after a storm event.

Upon completion of the deep test its and infiltration testing proposed under the response to Comment 2, the calculations and supporting data will be provided to town staff.

16. Provide flared end and countermeasures at the roof drain outfalls to the detention basin to protect against erosion.

Sheet EC-2 has been revised to depict stone aprons at the outfall locations of the 8-inch roof drains. The aprons will be formed in the field using 3 cubic feet of ¾ inch stone.

17. Provide evidence of a right to pass and repass onto the adjacent property to the north of the proposed development. Also, provide copies of any approvals obtained from the Connecticut Department of Transportation or the Office of State Transportation Administration for the project file.

Find attached the current OSTA Certificate for the PAD Master Plan. The property owner is in the process of finalizing the legal paperwork for the rights to use the adjacent CHUBB/Tower Park driveway. The developer will need to obtain an Encroachment Permit from CTDOT District Office for the utility work in Route 10.

18. Provide a detail for the detention / infiltration basin.

The Grading Plan show the size of the basin, side slopes, location of outfall weir, etc. For further clarity, a section of the detention/infiltration basin has been added to Sheet SD-4.

19. Some of the pipes from the subsurface chambers to catch basins / manholes are flowing upgradient although they are “downstream”. The design appears that the chambers will fill up prior to entering the larger pipe network. Consider providing an overflow at the surface for the downspouts in the event of a backup of runoff. Inspection ports should also be provided for these chambers.

The pipes leading from the chambers to an adjacent storm drain structure are

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indeed overflow pipes, set at flow lines that will require the chambers fill prior to discharge and subsequent conveyance to the larger pipe network and/or detention/infiltration basin. In the event of back-up of flow in the chambers, a detail has been added to Sheet SD-3 showing the design for an overflow for the downspouts connections for all roof leaders.

Sheet GR-1 has been revised to depict inspection ports in the ends of each row of underground chambers.

20. If not already done, please coordinate with the Fire Marshal to review emergency access requirements for the site.

The plans have been forwarded to the Fire Marshal for review and comment.

**Talcott Mountain Self Storage - Simsbury, CT
Water Quality Volume Size Calculations**

April 28, 2021

Minimum-Recommended Water Quality Volume (WQV)

Watershed	Total Area (Ac)	Impervious Area - I (Ac)	Impervious (%)	Runoff (R)	Min. Rec. WQV (ac-ft)	Min. Rec. WQV (Cu.Ft.)
WS-P-E-DET	3.53	1.30	36.8	0.3814	0.11221	4,888

$$WQV = \frac{(1")(R)(A)}{12}$$

WQV = water quality volume (ac-ft)

R = volumetric runoff coefficient
0.05+0.009(I)

I = percent impervious cover

Provided Water Quality Volume

Water Quality Basin (Total Volume)

Watershed	Elevations (Ft.)	Area (Sq. Ft.)	Avg. Area (Sq. Ft.)	Avg. Depth (FT)	Avg. Vol (Cu. Ft.)	Total Provided WQV (Cu. Ft.)
A (WQB#1)	190.0	3845				17,620
			4506	1.00	4506	
	191.0	5166				
			5855	1.00	5855	
	192.0	6543				
		7260	1.00	7260		
	193.0	7977				

Provided Water Quality Volume

Water Quality Basin (Forebay Volume)

Watershed	Elevations (Ft.)	Area (Sq. Ft.)	Avg. Area (Sq. Ft.)	Avg. Depth (FT)	Avg. Vol (Cu. Ft.)	Provided WQV (Forebay) (Cu. Ft.)
A (WQB#1)	190.0	528				1,575
			653	1.00	653	
	191.0	777				
			923	1.00	923	
	192.0	1068				

* requires 25% of Total WQV for Infiltration Basin = 1,222 Cu. Ft.

Hydraflow Rainfall Report

Hydraflow Hydrographs by Intelisolve v9.1

Wednesday, Apr 28, 2021

Return Period (Yrs)	Intensity-Duration-Frequency Equation Coefficients (FHA)			
	B	D	E	(N/A)
1	20.6018	3.9000	0.7306	-----
2	23.3393	3.6000	0.7167	-----
3	0.0000	0.0000	0.0000	-----
5	29.9317	3.7000	0.7174	-----
10	36.1004	3.9000	0.7226	-----
25	42.8187	3.8000	0.7187	-----
50	47.2025	3.6000	0.7124	-----
100	53.4440	3.7000	0.7151	-----

File name: SIMSBURY (IN PER HR).IDF

$$\text{Intensity} = B / (Tc + D)^E$$

Return Period (Yrs)	Intensity Values (in/hr)											
	5 min	10	15	20	25	30	35	40	45	50	55	60
1	4.17	3.01	2.41	2.03	1.76	1.57	1.42	1.30	1.20	1.12	1.05	0.99
2	4.99	3.59	2.87	2.42	2.11	1.88	1.70	1.56	1.44	1.35	1.26	1.19
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.34	4.58	3.66	3.09	2.69	2.40	2.17	1.99	1.84	1.72	1.61	1.52
10	7.44	5.39	4.32	3.64	3.18	2.83	2.56	2.35	2.17	2.02	1.90	1.79
25	8.97	6.49	5.20	4.39	3.83	3.41	3.09	2.83	2.62	2.44	2.29	2.16
50	10.19	7.35	5.88	4.96	4.33	3.86	3.50	3.21	2.97	2.77	2.60	2.45
100	11.38	8.22	6.58	5.56	4.85	4.32	3.91	3.59	3.32	3.10	2.90	2.74

Tc = time in minutes. Values may exceed 60.

Precip. file name: simsbury.pcp

Storm Distribution	Rainfall Precipitation Table (in)							
	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	0.00	3.31	0.00	4.41	5.32	6.58	7.50	8.52
SCS 6-Hr	0.00	2.23	0.00	2.87	3.40	4.13	4.66	5.00
Huff-1st	0.00	1.55	0.00	2.75	4.00	5.38	6.50	8.00
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	1.75	0.00	2.80	3.90	5.25	6.00	7.10



NOAA Atlas 14, Volume 10, Version 3
 Location name: Simsbury, Connecticut, USA*
 Latitude: 41.8805°, Longitude: -72.8009°
 Elevation: 185.86 ft**
 * source: ESRI Maps
 ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

[PF tabular](#) | [PF graphical](#) | [Maps & aerals](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour) ¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	4.18 (3.22-5.40)	5.00 (3.84-6.47)	6.35 (4.86-8.23)	7.46 (5.69-9.73)	9.00 (6.65-12.3)	10.2 (7.37-14.2)	11.4 (8.02-16.5)	12.7 (8.53-18.9)	14.6 (9.46-22.6)	16.2 (10.2-25.5)
10-min	2.96 (2.27-3.82)	3.55 (2.72-4.58)	4.50 (3.45-5.84)	5.29 (4.03-6.89)	6.37 (4.71-8.71)	7.19 (5.21-10.0)	8.05 (5.68-11.7)	9.00 (6.04-13.4)	10.4 (6.70-16.0)	11.5 (7.25-18.1)
15-min	2.32 (1.78-3.00)	2.78 (2.14-3.59)	3.52 (2.70-4.57)	4.14 (3.16-5.41)	5.00 (3.69-6.83)	5.64 (4.09-7.88)	6.31 (4.45-9.16)	7.06 (4.74-10.5)	8.12 (5.26-12.6)	8.98 (5.68-14.2)
30-min	1.57 (1.21-2.02)	1.88 (1.45-2.43)	2.40 (1.84-3.11)	2.83 (2.15-3.69)	3.41 (2.52-4.66)	3.86 (2.80-5.39)	4.32 (3.05-6.27)	4.83 (3.24-7.20)	5.56 (3.60-8.60)	6.16 (3.90-9.72)
60-min	0.988 (0.760-1.27)	1.19 (0.913-1.54)	1.52 (1.16-1.97)	1.79 (1.36-2.34)	2.16 (1.60-2.96)	2.45 (1.77-3.42)	2.74 (1.93-3.98)	3.07 (2.06-4.57)	3.53 (2.29-5.46)	3.91 (2.47-6.18)
2-hr	0.636 (0.492-0.816)	0.763 (0.590-0.980)	0.970 (0.748-1.25)	1.14 (0.875-1.48)	1.38 (1.03-1.88)	1.56 (1.14-2.17)	1.74 (1.24-2.54)	1.96 (1.32-2.91)	2.29 (1.49-3.53)	2.56 (1.63-4.03)
3-hr	0.488 (0.379-0.623)	0.586 (0.455-0.749)	0.746 (0.577-0.957)	0.879 (0.676-1.14)	1.06 (0.795-1.44)	1.20 (0.880-1.67)	1.34 (0.965-1.96)	1.52 (1.02-2.25)	1.79 (1.16-2.74)	2.02 (1.28-3.16)
6-hr	0.307 (0.240-0.390)	0.372 (0.291-0.473)	0.479 (0.373-0.611)	0.567 (0.439-0.728)	0.689 (0.519-0.933)	0.779 (0.577-1.08)	0.877 (0.635-1.28)	0.998 (0.676-1.47)	1.19 (0.775-1.82)	1.35 (0.863-2.11)
12-hr	0.187 (0.147-0.236)	0.231 (0.181-0.291)	0.302 (0.237-0.382)	0.361 (0.281-0.460)	0.442 (0.335-0.596)	0.501 (0.374-0.695)	0.567 (0.414-0.825)	0.650 (0.442-0.953)	0.781 (0.511-1.19)	0.896 (0.573-1.39)
24-hr	0.110 (0.087-0.137)	0.138 (0.109-0.173)	0.184 (0.145-0.231)	0.222 (0.174-0.281)	0.274 (0.210-0.369)	0.312 (0.235-0.432)	0.355 (0.262-0.517)	0.411 (0.280-0.599)	0.500 (0.328-0.758)	0.580 (0.372-0.896)
2-day	0.061 (0.049-0.076)	0.078 (0.063-0.098)	0.106 (0.084-0.133)	0.130 (0.102-0.163)	0.161 (0.124-0.217)	0.184 (0.140-0.255)	0.211 (0.157-0.308)	0.246 (0.168-0.357)	0.304 (0.200-0.459)	0.357 (0.230-0.549)
3-day	0.045 (0.036-0.055)	0.057 (0.046-0.071)	0.078 (0.062-0.097)	0.095 (0.075-0.119)	0.118 (0.091-0.158)	0.135 (0.103-0.186)	0.154 (0.116-0.225)	0.180 (0.124-0.261)	0.224 (0.148-0.337)	0.264 (0.170-0.405)
4-day	0.036 (0.029-0.045)	0.046 (0.037-0.057)	0.063 (0.050-0.078)	0.076 (0.061-0.095)	0.095 (0.074-0.127)	0.108 (0.083-0.149)	0.124 (0.093-0.180)	0.145 (0.099-0.209)	0.180 (0.119-0.270)	0.212 (0.137-0.325)
7-day	0.025 (0.020-0.030)	0.031 (0.025-0.038)	0.042 (0.034-0.052)	0.051 (0.040-0.063)	0.063 (0.049-0.083)	0.071 (0.055-0.098)	0.081 (0.061-0.118)	0.095 (0.065-0.136)	0.117 (0.078-0.175)	0.137 (0.089-0.209)
10-day	0.020 (0.016-0.025)	0.025 (0.020-0.031)	0.033 (0.026-0.040)	0.039 (0.032-0.049)	0.048 (0.038-0.064)	0.055 (0.042-0.074)	0.062 (0.047-0.089)	0.072 (0.050-0.103)	0.088 (0.058-0.131)	0.102 (0.066-0.155)
20-day	0.015 (0.012-0.018)	0.017 (0.014-0.021)	0.021 (0.017-0.026)	0.025 (0.020-0.030)	0.029 (0.023-0.038)	0.033 (0.025-0.044)	0.036 (0.027-0.051)	0.041 (0.029-0.059)	0.049 (0.033-0.072)	0.056 (0.036-0.084)
30-day	0.012 (0.010-0.015)	0.014 (0.011-0.017)	0.017 (0.014-0.020)	0.019 (0.015-0.023)	0.022 (0.017-0.028)	0.024 (0.019-0.032)	0.027 (0.020-0.037)	0.030 (0.021-0.042)	0.035 (0.023-0.051)	0.039 (0.025-0.058)
45-day	0.010 (0.008-0.012)	0.011 (0.009-0.014)	0.013 (0.011-0.016)	0.015 (0.012-0.018)	0.017 (0.013-0.022)	0.019 (0.014-0.024)	0.020 (0.015-0.028)	0.022 (0.016-0.031)	0.025 (0.017-0.036)	0.027 (0.018-0.041)
60-day	0.009 (0.007-0.011)	0.010 (0.008-0.012)	0.011 (0.009-0.014)	0.013 (0.010-0.015)	0.014 (0.011-0.018)	0.016 (0.012-0.020)	0.017 (0.012-0.023)	0.018 (0.013-0.025)	0.020 (0.013-0.029)	0.021 (0.014-0.032)

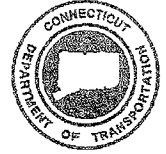
¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

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PF graphical



STATE OF CONNECTICUT
DEPARTMENT OF TRANSPORTATION



2800 BERLIN TURNPIKE, P.O. BOX 317546
NEWINGTON, CONNECTICUT 06131-7546
Phone: (860) 594-3020

December 16, 2015

Mr. Mark Greenberg
Infinity IV, LLC
686 Main Street, Suite 200
Watertown, CT 06795

Dear Mr. Greenberg:

Subject: Town of Simsbury
CL&P Subdivision Expansion (Garden Homes - Masterplan) Traffic Investigation Report No.128-1504-01
CL&P Subdivision Expansion (Garden Homes - Phase1) Traffic Investigation Report No. 128-1511-01

Enclosed are copies of Traffic Investigation Report (TIR) Nos. 128-1504-01 and 128-1511-01 approved on December 16, 2015 detailing conditions for the issuance of a Certificate.

In accordance with Section 14-311 of the General Statutes, a building/foundation permit cannot be obtained from the Town until a Certificate is issued.

Except as noted in TIR No. 128-1511-01, a Certificate will not be issued by this office until:

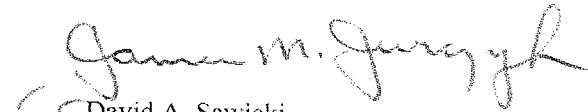
1. TIR Nos. 128-1504-01 has been recorded on the municipal land records in accordance with Condition No. 25 **and this office has been provided with evidence of recording (i.e., copy of recorded report); and**
2. A bond, sufficient to cover the full cost of implementing the required improvements, has been posted with the Department of Transportation District 4 Office (Attn: Mr. James Lapan) at 359 South Main Street, Thomaston, CT 06787, (860) 585-2795, as noted in Condition No. 24 of TIR No. 128-1504-01 **and this office has been so notified, in writing, by the District Office.**

As noted in TIR No. 128-1511-01, a certificate may be issued to allow limited building or foundation permits to be issued for Garden Homes - Phase 1 of the development providing Condition No. 16 of that TIR has been completed **and this office has been provided with written evidence as such.**

Additionally, an encroachment permit must be obtained from the District Office prior to performing any work within the state highway right-of-way. **An encroachment permit will not be issued until the certificate has been issued and filed on the municipal land records in accordance with the attached procedure; the easements noted in Condition Nos. 17, 21, and 22 have been granted; and detailed construction plans have been reviewed and approved by the District Office.** Enclosed is the Procedure

for Transferring Property and/or Easements to the State of Connecticut, to satisfy Condition Nos. 17, 21, and 22. The approvals will expire on December 16, 2018, unless all the requirements are satisfied or an extension of the approvals is granted by this office.

Sincerely,



David A. Sawicki
Executive Director
Office of the State Traffic Administration

Enclosures

Copy to: Chief Peter N. Ingvertsen
Mr. Henry Miga - Please confirm Certificate issuance with OSTA. Note Condition 19 of
TIR No. 128-1511-01
Planning Commission
Zoning Commission
Mr. Mark Vertucci, P.E., PTOE
Mr. Lyle Wray

Simsbury
Town of Simsbury Town Clerk
Town Hall
933 Hopmeadow Street
Simsbury, CT 060701822
Phone Number: (860)658-3243

Official Receipt: 2016-00000096
Printed on 01/11/2016 at 09:47:56 AM
By: 54 on STH-COTT-REG1

STATE OF CONNECTICUT DOT
DEPARTMENT OF TRANSPORTATION
2800 BELRIN TPK PO BOX 317546
NEWINGTON CT 6131-7546

Date Recorded: 01/11/2016

Instrument ID	Recorded Time	Amount
Bk 896 Pg 982	09:43:26 AM	\$83.00
TRAFFIC REPORT		
INFINITY IV LLC		
TO:STATE OF CONNECTICUT, DEPARTMENT O...		
Accounts		Amount
RECORDING FEES		\$40.00
H D P A REC SI T C		\$1.00
H D P A REC CT IIB		\$2.00
C I A REC SI G F		\$1.00
C I A REC L O C I P		\$3.00
C I A REC CT		\$36.00

Total Due: \$83.00
Paid By Cash: \$100.00
Change Tendered: (\$17.00)

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